

Multiphase Flow in the Subsurface

- Flow of a Light Nonaqueous Phase Liquid (LNAPL)

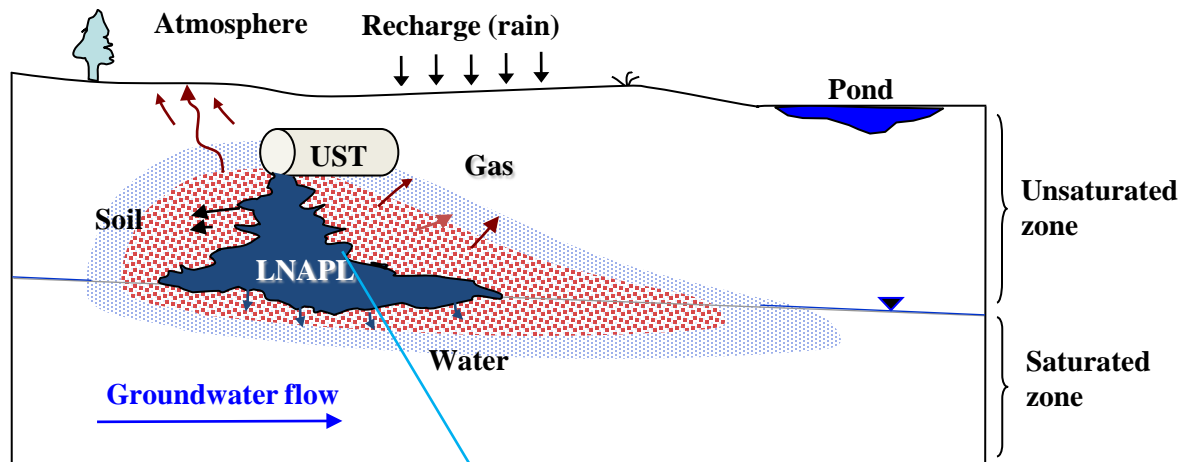
March 29, 2011

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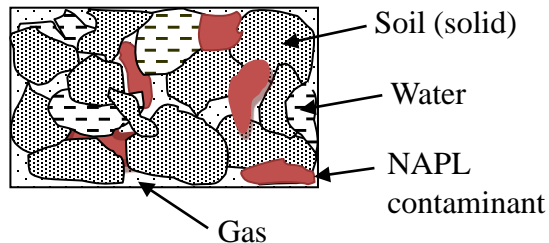
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Introduction to Multiphase Flow

- **Multiphase flow means “the simultaneous movement of multiple phases, such as water, air, non-aqueous phase liquid (NAPL), through porous media.”**



*Pore-scale
soil matrix*

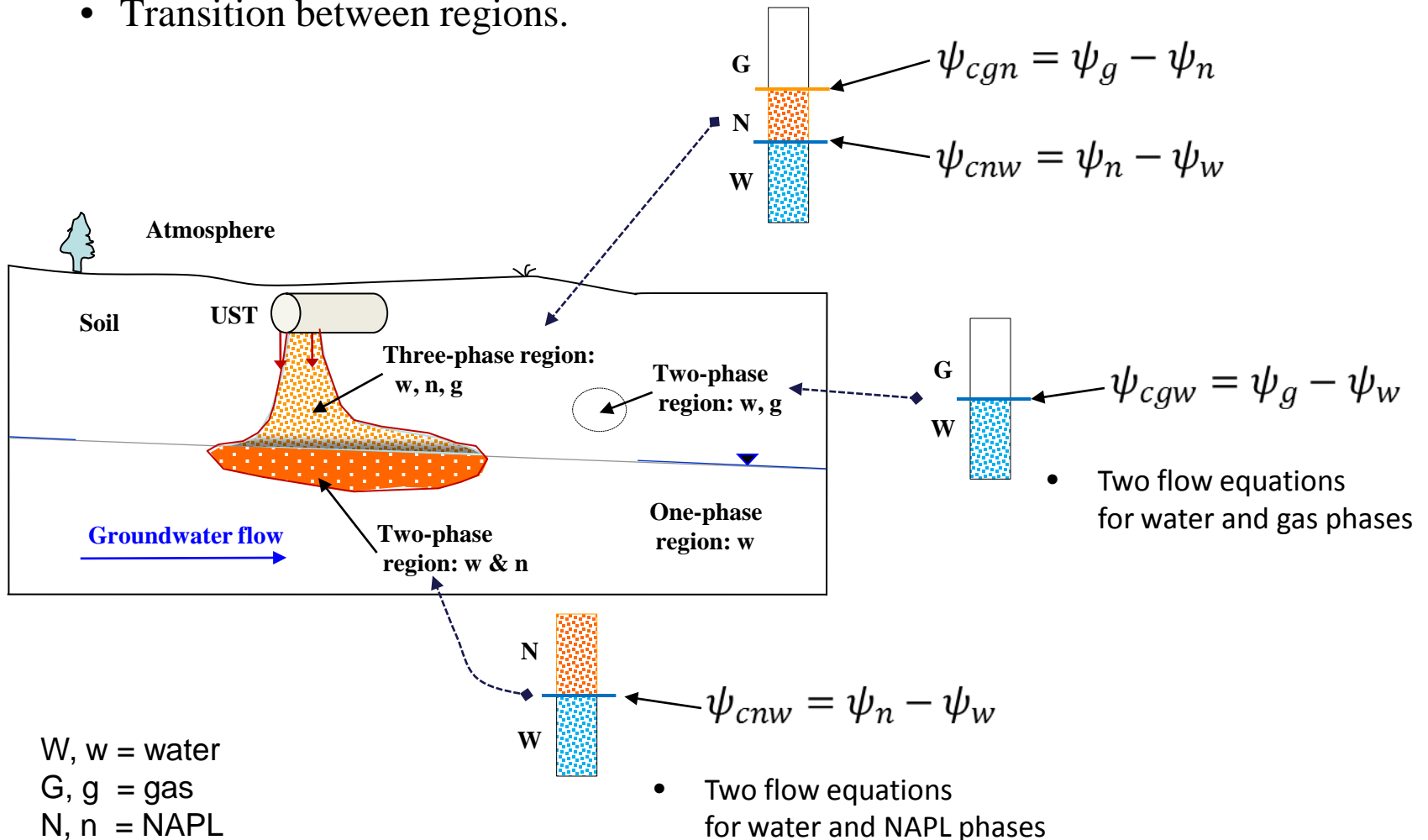


Capillary Pressure between Phases

■ Numerical difficulty

- Transition between regions.

- Three flow equations for water, gas and NAPL phases



Mathematical Approach for Multiphase Flow

- **Governing equations: Groundwater, gas, and NAPL**

$$\frac{\partial(\phi s_f \rho_f)}{\partial t} + \nabla \cdot (\rho_f \mathbf{q}_f) = I_f + Q_f$$

$$\mathbf{q}_f = -\frac{\mathbf{k} k_{rf}}{\mu_f} \rho_{rw} g \cdot \left(\nabla \psi_f - \left(\frac{\rho_f}{\rho_{rw}} \right) e_z \right)$$

$$\psi_f = P_f / \rho_{RW} g$$

ρ_{RW} = the reference water density
 $f = w$ (water), g (gas), and n (NAPL).

Capillary pressure

$$\psi_{cgw} = \psi_g - \psi_w,$$

$$\psi_{cnw} = \psi_n - \psi_w,$$

$$\psi_{cgn} = \psi_g - \psi_n$$

- C. Pressure (ψ_c)-Saturation (s_f)-R. Permeability (k_{rf}) Relations
→ Nonlinear and very complicated to solve the equations.

C. Pressure-Saturation-R. Permeability (1)

▪ cP-S-kr relationships

- Brooks-Corey law (1964)

$$s_{we} = \frac{1 - s_n - s_{wr}}{1 - s_{nr} - s_{wr}} = \left(\frac{\psi_d}{\psi_{cnw}} \right)^\lambda, \quad \psi_d > \psi_{cnw}$$

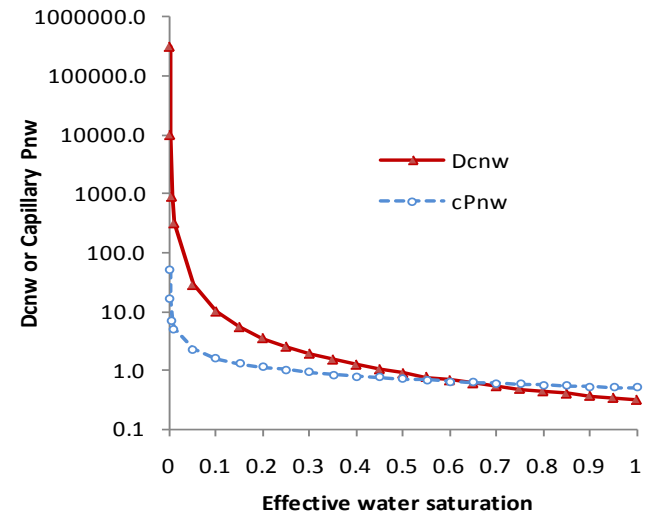
$$k_{rw} = s_{we}^{\frac{2+3\lambda}{\lambda}}$$

$$k_{ro} = (s_{te} - s_{we})^2 \left(s_{te}^{\frac{2-\lambda}{\lambda}} - s_w^{\frac{2-\lambda}{\lambda}} \right)$$

- ψ_d is the air-entry pressure head of the air-water system.
- λ is the pore size distribution.

Brooks-Corey law

Size index	2
Entry Pr. (Pd)	0.5099
Residual Sw	0.1
Residual Sn	0.1



$$\nabla \psi_{cnw} = \frac{d\psi_{cnw}}{ds_n} \nabla s_n = D_{cnw} \nabla s_n$$

C. Pressure-Saturation-R. Permeability (2)

▪ cP-S-kr relationships

- van Genuchten law (1980)

$$s_{we} = [1 + (\alpha\beta_{nw}\psi_{nw})^n]^{-m} \quad \psi_{nw} > 0$$

$$s_{we} = 1 \quad \psi_{nw} \leq 0$$

$$s_{te} = [1 + (\alpha\beta_{gn}\psi_{gn})^n]^{-m} \quad \psi_{gn} > 0$$

$$s_{te} = 1 \quad \psi_{gn} \leq 0$$

$$s_t = s_w + s_n$$

s_t = Total liquid saturation

$$m = 1 - 1/n$$

- α (L^{-1}) and n (dimensionless) are empirical parameters describing soil media
- β_{gn} and β_{nw} are the scaling factors

Three-Phase Systems in the Shallow Aquifer

- **Mobile phases: Water and NAPL**
- **Constant pressure head: Gas**
 - The soil gas in the unsaturated zone is connected to the atmosphere.
 - The gas movement has negligible impacts on the movement of water and NAPL.

$$\frac{\partial (\phi \rho_w (1 - s_n - s_g))}{\partial t} = \nabla \cdot \left(\frac{k \rho_w k_{rw}}{\mu_w} \rho_{RW} g \left(\nabla \psi_w + \frac{\rho_w}{\rho_{RW}} \mathbf{e}_z \right) \right) + Q_w$$

$$\frac{\partial (\phi \rho_n s_n)}{\partial t} = \nabla \cdot \left(\frac{k \rho_n k_{rn}}{\mu_n} \rho_{RW} g \left(\nabla (\psi_w + \psi_{cnw}) + \frac{\rho_n}{\rho_{RW}} \mathbf{e}_z \right) \right) + I_n + Q_w$$

$$s_w + s_g + s_n = 1.$$

$$\psi_n = \psi_w + \psi_{cnw}$$

- Primary variables: ψ_w and s_n
- Secondary variables: ψ_n, s_w, s_g


Water-NAPL Two-Phase System

- **Mobile phases: Water and NAPL**
- **No gas phase**
- **Example: CO₂ injection in deep geological systems**

$$s_w + s_n = 1.$$

$$\frac{\partial(\phi\rho_w s_n)}{\partial t} = \nabla \cdot \left(\frac{k\rho_w k_{rw}}{\mu_w} \rho_{RW} g \left(\nabla\psi_w + \frac{\rho_w}{\rho_{RW}} \mathbf{e}_z \right) \right) + Q_w$$

$$\frac{\partial(\phi\rho_n s_n)}{\partial t} = \nabla \cdot \left(\frac{k\rho_n k_{rn}}{\mu_n} \rho_{RW} g \left(\nabla(\psi_w + \psi_{cnw}) + \frac{\rho_n}{\rho_{RW}} \mathbf{e}_z \right) \right) + I_n + Q_w$$


$$\nabla\psi_{cnw} = \frac{d\psi_{cnw}}{ds_n} \nabla s_n = D_{cnw} \nabla s_n$$

Numerical Techniques

- **Global implicit scheme**

- Solves multiphase flow equations simultaneously.
- Generates a non-symmetric global matrix.

$$\begin{bmatrix} \psi_w & \cdots & s_n \\ \vdots & \ddots & \vdots \\ \psi_w & \cdots & s_n \end{bmatrix} \begin{bmatrix} \psi \\ s \end{bmatrix} = \begin{bmatrix} \psi_0 \\ s_0 \end{bmatrix}$$

- **Upstream weighting scheme (Upwind scheme)**

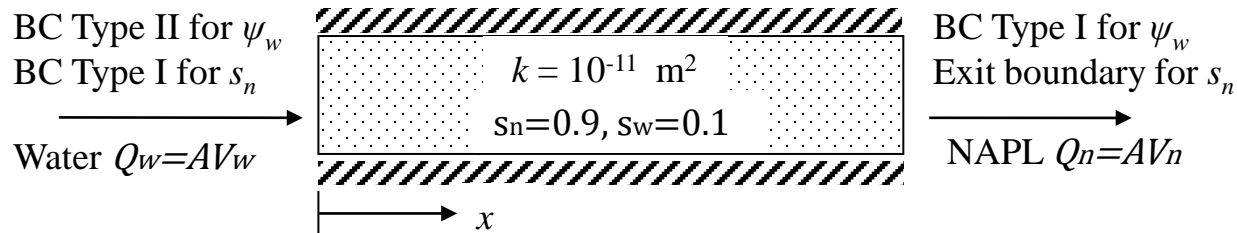
- Relative permeability is evaluated based on a flow direction.

- **Sparse matrix solvers**

- Iterative matrix solver: IML++
 - Failed when the global implicit scheme is used.
- Direct matrix solver: Pardiso solver
 - Works good with the global implicit scheme.

Buckley-Leverett Problem

- **Buckley-Leverett problem represents a linear water-flood of a petroleum reservoir in a one-dimensional, horizontal domain.**
 - The pore spaces of the domain is initially filled with a NAPL, i.e., liquid oil.



Properties	Values
Boundary condition	
Water influx at $x=0$ m Water pressure at $x=300$ m	$v_w = 0.01 \text{ m/s}$, BC Type II $p_w = 2.9 \text{ m H}_2\text{O}$, BC Type I
NAPL saturation at $x=0$ m (s_w at $x=0$ m)	$s_n = 0.1$, BC Type I ($s_w = 0.9$, BC Type I)
Initial condition	
Water saturation	$s_w = 0.1$
NAPL saturation	$s_n = 0.9$

Darcy velocity = 0.01 m/s

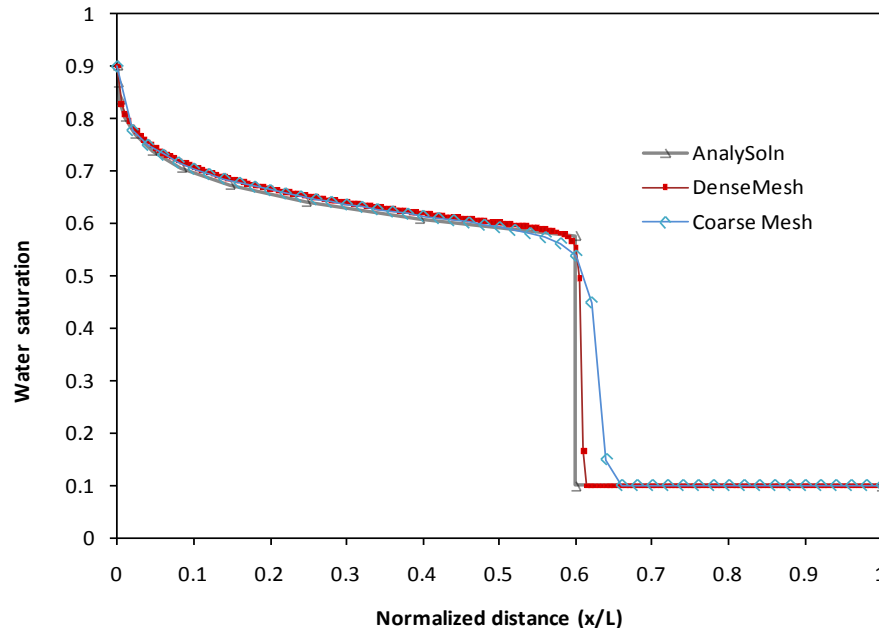
Buckley-Leverett Problem (contd.)

- Parameters

Properties	Values	Comment
Soil		
Intrinsic permeability	10^{-11} m^2	
Porosity	0.3	
Pore size distribution index	2.0	Brook-Corey law
Water residual saturation	$s_{wr} = 0.1$	
NAPL residual saturation	$s_{nr} = 0.1$	
Fluid		
Water density	$\rho_w = 1000 \text{ kg/m}^3$	
NAPL (oil) density	$\rho_n = 900 \text{ kg/m}^3$	
Water viscosity	$\mu_w = 0.001 \text{ Pa s}^{-1} \text{ (kg/ms)}$	
NAPL(oil) viscosity	$\mu_n = 0.005 \text{ Pa s}^{-1} \text{ (kg/ms)}$	

Buckley-Leverett Problem (Results)

- **Comparison of water saturation profiles**
 - Semi-analytical solution vs. TechFlowMP results
 - Coarse and dense meshes



Location of the water front

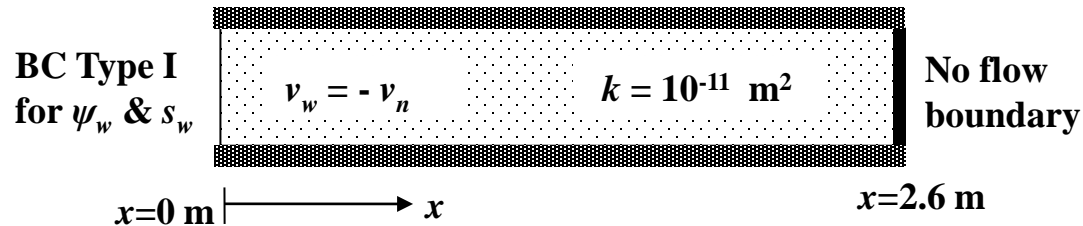
$$x_f = \frac{Q_w t}{A\phi} \left(\frac{df_w}{ds_w} \right)$$

$$f_w = \frac{1}{1 + \frac{k_{rn} \mu_w}{k_{rw} \mu_n}}$$

Domain size, Length	L = 5 m	
Space step size, SD-A	$\Delta x = 0.1$ m	Coarse grid
Space step size, SD-B	$\Delta x = 0.025$ m	Dense grid

McWhorter-Sunada Problem

- The flows of water and NAPL are initiated by the capillary pressure between two phases in a domain.



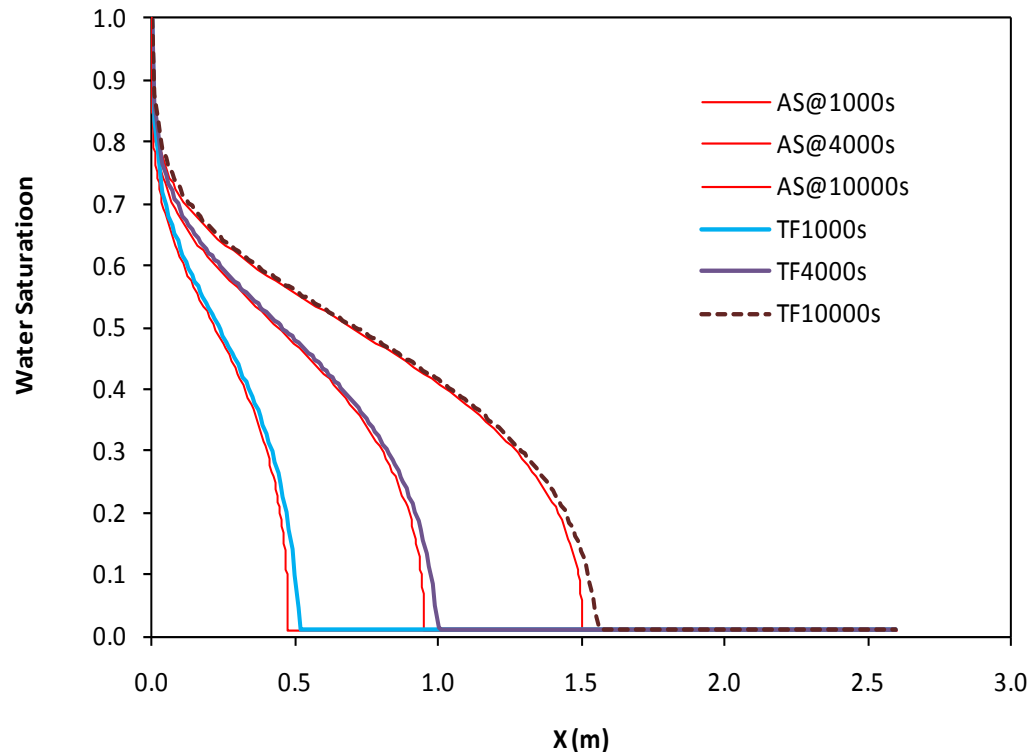
Properties	Values
Boundary condition	
Water pressure ($x=0 \text{ m}, t$)	$\psi_w = 19.885 \text{ m H}_2\text{O}$, BC Type I
Water pressure ($x=5 \text{ m}, t$)	No flux/flow boundary
NAPL saturation ($x=0 \text{ m}, t$)	$s_n = 0.$, BC Type I
(Water saturation ($x=0 \text{ m}, t$))	($s_w = 1.$, BC Type I)
NAPL saturation ($x=5 \text{ m}, t$)	No flow boundary
Initial condition	
Water saturation ($x, t=0$)	$s_w = 0.01$
NAPL saturation ($x, t=0$)	$s_n = 0.99$
Water pressure (x, t)	$\psi_w = 19.885 \text{ m H}_2\text{O}$ ($P_w = 195000 \text{ Pa}$)

McWhorter-Sunada Problem (contd.)

Properties	Values	Remark
Soil		
Soil intrinsic permeability	10^{-11} m^2	
Porosity	0.3	
Pore size distribution index	2	Brook-Corey law 1 mH ₂ O=9806.65Pa
Entry pressure, P_d	5000 Pa ($\psi_w=0.5099 \text{ mH}_2\text{O}$)*	
Water residual saturation	$s_{wr} = 0.$	
NAPL residual saturation	$s_{nr} = 0.$	
Fluid		
Water density	$\rho_w = 1000 \text{ kg/m}^3$	
NAPL (oil) density	$\rho_n = 1000 \text{ kg/m}^3$	
Water viscosity	$0.001 \text{ Pa s}^{-1} (= \text{kg/m s})$	
NAPL(oil) viscosity	$0.001 \text{ Pa s}^{-1} (= \text{kg/m s})$	
Domain and space discretization		
Domain size, Length	$L = 2.6 \text{ m}$	260 elements
Space step size	$\Delta x = 0.01 \text{ m}$	
Water viscosity	$0.001 \text{ Pa s}^{-1} (= \text{kg/m s})$	
NAPL(oil) viscosity	$0.001 \text{ Pa s}^{-1} (= \text{kg/m s})$	
Time discretization		
Simulation time	$T = 10,000 \text{ s}$	
Time step size	$\Delta t = 1 - 100 \text{ s (Max. 15 iterations)}$	

McWhorter-Sunada Problem (contd.)

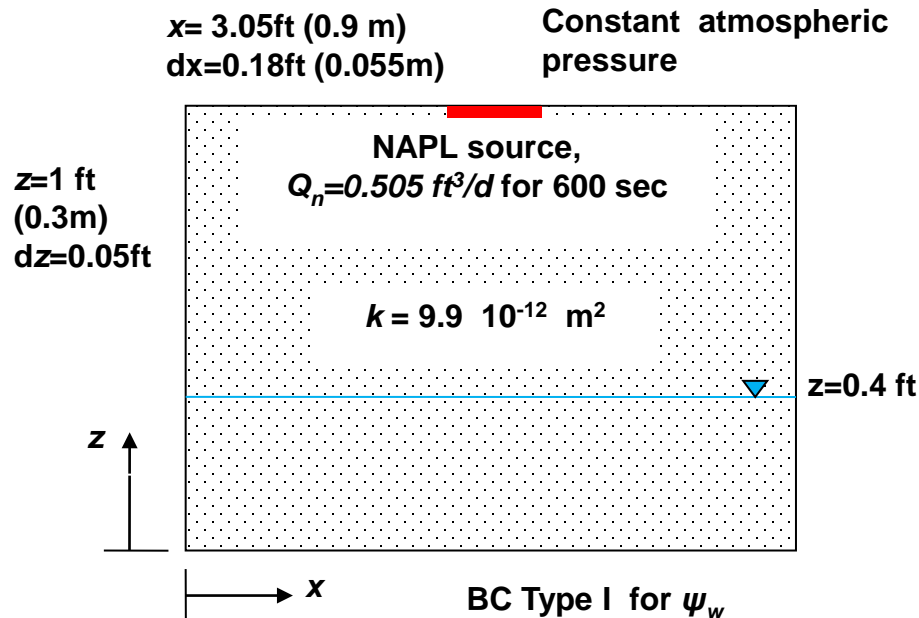
- **The change in water saturation over time**
 - Semi-analytical solutions vs. TechFlowMP results
 - The global implicit scheme, upwind scheme, and Pardiso solver are implemented.



NAPL Release at the Ground Surface

- **NAPL's release into the variably saturated zone.**

- Three phases: water, gas, and NAPL.
- A NAPL is released for 600 sec.



Initial condition

- Water: Variable s_w
- NAPL: $s_n = 0$ at $t = 0$ sec
- Water head: $\psi_w = 0.4 \text{ ft H}_2\text{O}$

Domain and space discretization

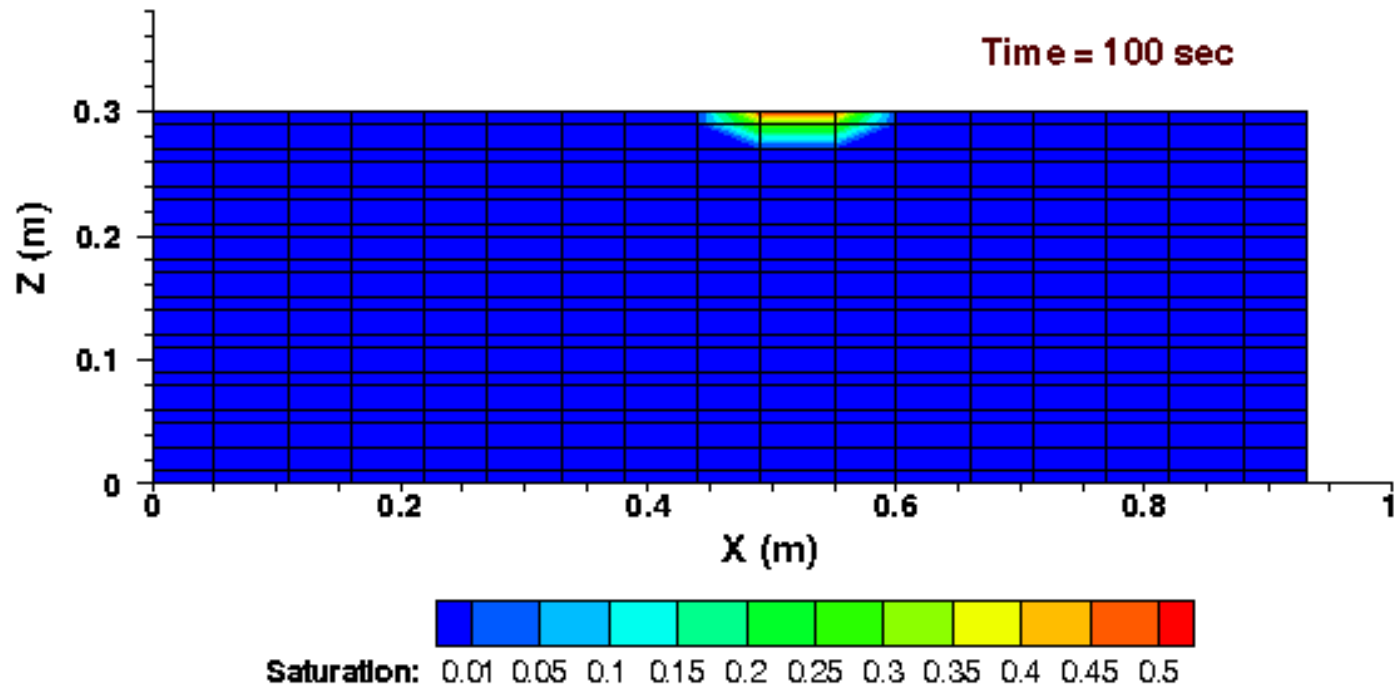
- $X = 93 \text{ cm}$: $\Delta x = 0.18 \text{ ft (5.47 cm)}$
- $Z = 30.48 \text{ cm}$: $\Delta z = 0.05 \text{ ft (1.524 cm)}$

Time discretization

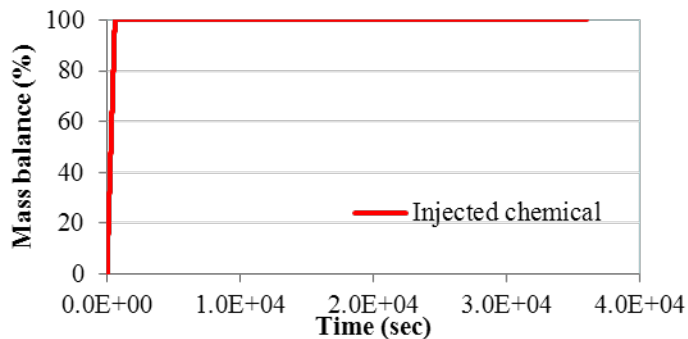
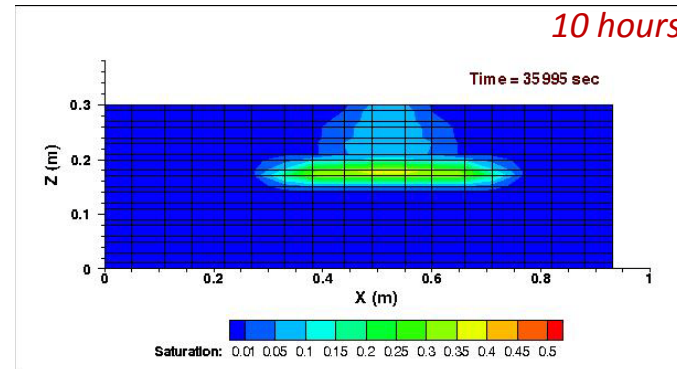
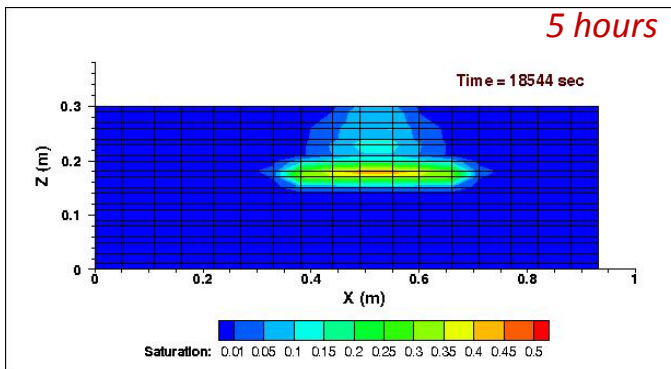
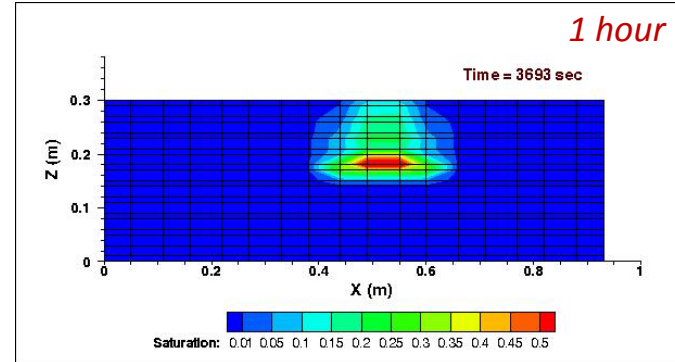
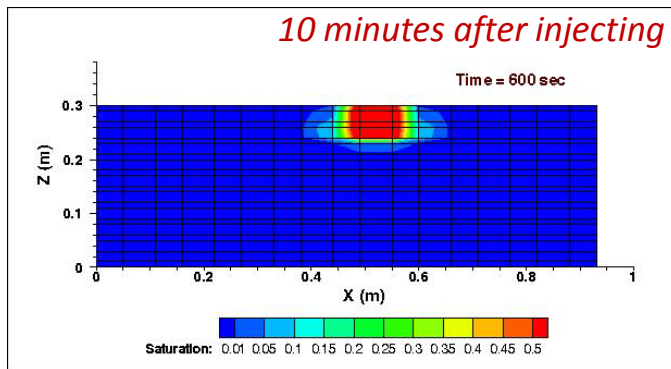
- Simulation time: $T = 10 \text{ hrs}$
 $(\Delta t = 0.01 - 8 \text{ sec})$

NAPL Release at the Ground Surface (contd.)

NAPL's spreading with time.



NAPL Release at the Ground Surface (contd.)

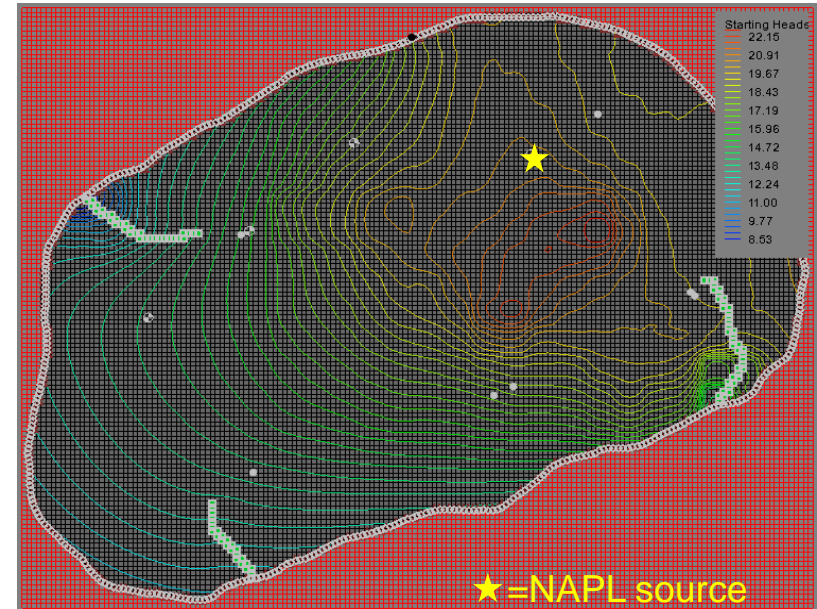


- The spreading of the released NAPL is expected to be completely within a relatively short period of time.
- The immobilized NAPL becomes a long-lasting contaminant source.

GW Pollution in the Hadnot Point Industrial Area

- **HPIA, Camp Lejeune, NC.**

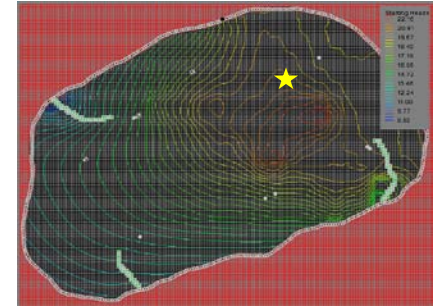
Parameters	Description
Domain size	Length in x-axis: 8200.0 ft ($\Delta x=50$ ft) Length in y-axis: 6450.0 ft ($\Delta y=50$ ft) Depth: from 7.47161 ft to -240.744 ft Origin: (X= 2497210.0 ft, Y=335640.0, Z=0.0)
Grid	Total number of rows (Cells i): 129 Total number of columns (Cells j): 164 Total number of layers (Cells k): 7 Number of nodes: 171,600 Number of cells: 148,092 (No. active cells: 99,352; inactive cells: 48,740)
Elevation	Number of elevation data: 148,092 Minimum value: -240.744 ft Maximum value: 7.47161 Mean: -98.2263, Median: -77.2142 Reference time: 12/30/1988
Stress period	240 (from 1/1/11942 to 1/1/1962 = 7305 days)



Application to GW Pollution in HPIA (contd.)

▪ NAPL at HPIA, Camp Lejeune, NC.

- Contaminant sources are immobilized NAPLs.
- The dissolution of the immobile NAPL and its transport in the whole domain will be investigated.
- The migration of the NAPL can be analyzed within a very limited region around the source area.



Thank you.

Questions?

References

- Brooks, R.H. and Corey, A.T., 1964. Hydraulic Properties of Porous Media. Hydrology Paper 3., 27 pp., Colorado State University, Fort Collins, Co.
- Helmig, R., 1997. Multiphase flow and transport processes in the subsurface : a contribution to the modeling of hydrosystems. Environmental engineering. Springer, Berlin ; New York, xvi, 367 p. pp.
- van Genuchten, M.T., 1980. A closed-form equation for predicting the hydraulic conductivity of unsaturated soils. Soil Science Society of America Journal, 44(5): 892-898.